Earthtec Testing & Engineering, P.C.

133 North 1330 West
Orem, Utah - 84057
Phone (801) 225-5711
Fax (801) 225-3363

1596 W. 2650 S. #108
Ogden, Utah - 84401
Phone (801) 399-9516
Fax (801) 399-9842

GEOTECHNICAL STUDY
CARNESECCA ORCHARDS ESTATES
1700 NORTH MAIN
MAPLETON, UTAH

Prepared By:

Earthtec
Testing and Engineering, P.C.

133 North 1330 West
Orem, Utah 84057
(801) 225-5711

Job No. 061567

Prepared for:

Chris Shurian
560 South 100 West, Suite 1
Provo, Utah 84601

June 30, 2006

TABLE OF CONTENTS **Page No.**

FIGURES

TABLES

1.0 INTRODUCTION

Earthtec has completed a geotechnical study for an approximately 5½ acre parcel located at about 1700 North Main Street in Mapleton, Utah. The general location of the site is shown on Figure No. 1, *Vicinity Map*, at the end of this report. This report presents our findings and conclusions.

The purposes of this study were to 1) evaluate the subsurface soil conditions at the site, 2) assess the engineering characteristics of the subsurface soils, and 3) provide geotechnical recommendations for general site grading and the design and construction of foundations, concrete floor slabs, miscellaneous concrete flatwork, and asphalt pavement sections. The scope of work completed for this study included field reconnaissance, subsurface investigation, field and laboratory soil testing, engineering analysis, and the preparation of this report.

2.0 CONCLUSIONS

The following is a brief summary of our findings and conclusions:

1. In the test pits we observed approximately 12 to 18 inches of topsoil at the surface. Subsurface soils we encountered were composed of Gravel (GM, GP, GP-GM) and Sand (SP-SM) extending to the maximum depths explored of about 9 to 11 feet below the existing surface. Groundwater was not encountered within the depths explored.

2. Topsoil and any disturbed or other unsuitable soils should be completely removed from below foundation, floor slab, exterior concrete flatwork, and pavement areas.

3. Conventional strip and spread footings may be used to support proposed residences within this development. We recommend that foundations be constructed entirely on undisturbed, uniform, native gravel soils, or entirely on a minimum 18 inches of structural fill placed on undisturbed native soils. Footings constructed as described above may be designed for a maximum bearing capacity of 2,000 psf. More details regarding foundation design can be found in Section 10.0 of this report.

These findings and conclusions should not be relied upon without reading and consulting this entire report for a more detailed description of the geotechnical evaluation and recommendations contained herein.

3.0 PROPOSED CONSTRUCTION

We anticipate that development will include placing utilities, concrete flatwork in the form of curb, gutter, sidewalks, and driveways, and an asphalt concrete paved residential street, and the construction of single family residences.

We estimate that foundation loads for residences will not exceed 3 kips per linear foot for bearing walls and 150 pounds per square foot for floor slabs. If structural loads will be greater, our office should be notified so that we may review our recommendations and, if necessary, make modifications.

4.0 GENERAL SITE DESCRIPTION

At the time we conducted our subsurface explorations the site was a pasture vegetated with grass, weeds, and some trees around the perimeter. The site was relatively flat and was bounded on the north and south by houses and pasture, on the east by Main Street, and on the west by pasture.

5.0 SUBSURFACE INVESTIGATION

Under the direction of a qualified member of our geotechnical staff, a subsurface investigation was conducted at the site on June 13, 2006, by excavating 4 exploratory test pits to an approximate depth of 10 feet below the existing ground surface using a rubber tire backhoe. The approximate locations of the test pits are shown on Figure No. 2 at the end of this report. We understand that the developer did not yet own about 1/3 of the site on the west side thus we were unable to explore on that portion of the proposed development.

The soils exposed in the test pits were classified by visual examination following the guidelines of the Unified Soil Classification System (USCS). Disturbed bag samples of the subsurface soils were collected at various intervals in the test pits. These samples were transported to our Orem, Utah laboratory where they will be retained for 30 days following the date of this report and then discarded, unless a written request for additional holding time is received prior to the 30 day limit.

6.0 LABORATORY TESTING

From the samples collected in the test pits, representative samples were selected for laboratory testing to assess pertinent engineering properties and to aid in refining field classifications, if needed. Laboratory testing consisted of natural moisture content tests and mechanical gradation analyses. The following table summarizes the results of the laboratory testing. Test results are also shown on the enclosed test pit logs at the respective sample depths.

Table No. 1: Laboratory Test Results

TEST PIT NO.	DEPTH (ft.)	NATURAL MOISTURE (%)	GRAIN SIZE DISTRIBUTION (%)			SOIL TYPE
			GRAVEL #4	SAND	SILT/ CLAY #200	
TP-1	6	6	66	30	4	GP
TP-2	9	7	39	51	10	SP-SM
TP-3	2½	4	69	28	3	GP
TP-4	5	3	65	32	3	GP

7.0 SUBSURFACE CONDITIONS

7.1 Soil Types

We encountered topsoil at the surface of the test pits which we observed to extend about 12 to 18 inches in depth. Below the topsoil we encountered layers of Silty Gravel with sand (GM),

Poorly Graded Gravel with sand (GP), Poorly Graded Gravel with silt and sand (GP-GM), and an occasional layer of Poorly Graded Sand with silt and gravel (SP-SM) extending to the maximum depths explored of about 9 to 11 feet below the existing surface.

Graphical representations and detailed descriptions of the soils encountered are shown on Figure Nos. 3 through 6, *Test Pit Log* at the end of this report. The stratification lines shown on the logs represent the approximate boundary between soil units, the actual transition may be gradual. Due to potential natural variations inherent in soil deposits, care should be taken in interpolating between and extrapolating beyond exploration points. A key to the symbols and terms on the logs is presented on Figure No. 7, *Legend.*

7.2 Groundwater

Groundwater was not encountered in the test pits. Groundwater levels will fluctuate in response to the season, precipitation and snow melt, irrigation, and other on and off-site influences. Precisely quantifying these fluctuations would require long term monitoring.

8.0 SITE GRADING

8.1 General Site Grading

Unsuitable soils and vegetation should be removed from below foundation, floor slab, exterior concrete flatwork, and pavement areas to minimize the potential for distress and settlement. Unsuitable soils consist of topsoil, organic soils, undocumented fill, soft, loose, or disturbed native soils, and any other inapt materials. Topsoil was observed to extend approximately 12 to 18 inches in depth. The topsoil, including any soil containing roots larger than about ¼ inch in diameter, and any other unsuitable soils, should be completely removed beneath building, flatwork, and pavement areas.

The native gravel soils encountered in the test pits may be suitable for use as structural fill if found to meet the specifications given in Section 8.3, but they did contain cobbles and boulders. Soils which do not meet these specifications may be stockpiled for use as fill in landscape areas.

8.2 Temporary Excavations

For temporary excavations less than 5 feet in depth into the native soils or into structural fill, slopes should not be made steeper than 0.5:1.0 (horizontal:vertical). Temporary excavations extending up to 10 feet in depth should not be made steeper than 1:1. If unstable conditions or groundwater seepage are encountered, flatter slopes, shoring, or bracing may be required.

8.3 Fill Material

Regular structural fill should consist of imported material or native soils meeting the following requirements:

Maximum particle size:	4 inches
Percent retained on the 3/4 inch sieve (coarse gravel):	30 maximum
Percent passing the No. 200 sieve (fines):	15 maximum
Liquid Limit of fines:	35 maximum
Plasticity Index of fines:	15 maximum

In some situations, particles larger than 4 inches and/or more than 30 percent coarse gravel may be acceptable, however, compaction and compaction testing may be more difficult. As a result more strict quality controls measures than normally used may be required. Such measures may include using thinner lifts, and increased or full time observation of fill placement.

Utility trench fill below structures, concrete flatwork, and asphalt paving should consist of structural fill as defined above. In other areas, utility trenches can be backfilled with the native soil, however, native soils that are predominately fine grained may be time consuming to compact, due to difficulty adjusting the moisture content (thus using structural fill may be warranted). All backfill soil should meet the following requirements:

Maximum particle size:	4 inches
Liquid Limit of fines:	35 maximum
Plasticity Index of fines:	15 maximum

8.4 Fill Placement and Compaction

The thickness of each lift should be appropriate for the compaction equipment that is used. We recommend a maximum lift thickness of 4 inches for hand operated equipment, 6 inches for most "trench compactors", and 8 inches for larger rollers, unless it can be demonstrated by in-place density tests that the required compaction can be obtained throughout a thicker lift. The full thickness of each lift of structural fill placed should be compacted to at least the following percentages of the maximum dry density, as determined by ASTM D-1557:

In landscape areas not supporting structural loads:	90%
Less than 5 feet of fill below foundations, flatwork and pavements:	95%
Five or more feet of fill below foundations, flatwork and pavements:	98%

Generally, placing and compacting fill at a moisture content within 2% of the optimum moisture content, as determined by ASTM D-1557, will facilitate compaction. Typically, the further the moisture content is from optimum the more difficult it will be to achieve the required compaction.

Fill should be tested frequently during placement and early testing (initial lift) is recommended to demonstrate that placement methods and compaction efforts are achieving the required compaction. It is the contractor's responsibility to ensure that fill materials and compaction efforts are consistent so that tested areas are representative of the entire fill.

9.0 SEISMIC CONSIDERATIONS

9.1 Faulting

Based on published data no active faults are known to traverse the site and no surficial evidence of faulting was observed during our field investigation. The nearest mapped[1] fault trace is the Wasatch Fault located about ¾ miles northeast of the site.

9.2 Liquefaction Potential

The site is located in an area mapped by the Utah Geological Survey[2] as having moderate liquefaction potential. Liquefaction is a phenomenon where a soil loses intergranular strength due to an increase in soil pore water pressures during a dynamic event such as an earthquake. The potential for liquefaction is based on several factors, including 1) the grain size distribution of the soil, 2) the plasticity of the fine fraction of the soil (material passing the No. 200 sieve), 3) relative density of the soil, 4) earthquake strength (magnitude) and duration, and 5) overburden pressures. In addition, the soils must be saturated for liquefaction to occur. As a part of this investigation, the potential for liquefaction to occur in the soils we observed was assessed.

Loose, saturated sands are most susceptible to liquefaction, but soft, sensitive silt soils also have the potential to experience failure and movement during a seismic event. The soils observed in the test pits were not saturated and therefore are estimated to have low liquefaction potential.

9.3 IRC Seismic Design Category

The Seismic Design Categories in the International Residential Code (IRC) are based upon the short period design accelerations determined using the seismic provisions of the International Building Code (IBC) and the soil properties in the upper 100 feet of the soil profile. These

---

[1]Hecker, S., 1993, Quaternary Faults and Folds, Utah, Utah Geologic Survey, Bulletin 127.

[2]Liquefaction Potential Map, Utah Geological Survey, Public Information Series 25. 1994.

properties are determined from SPT blow counts and undrained shear strength measurements. The IBC code also states that "Where site specific data are not available to a depth of 100 feet, appropriate soil properties may be estimated by the registered design professional preparing the soils report...." We estimate that the majority of the soils encountered in the test pits have properties consistent with those defined by Site Class D.

The site is located at approximately 40.15 degrees latitude and about -111.58 degrees longitude. For Site Class D, Fa is 1.05 and S_{DS} = 0.79. The Seismic Design Category is D_1.

10.0 FOUNDATIONS

10.1 General

The foundation recommendations presented in this report are based on the soil conditions observed in the test pits, the results of laboratory testing of samples of the native soils, the site grading recommendations presented in this report, and the foundation loading conditions presented in Section 3.0, *Proposed Construction*, of this report. If loading conditions are significantly different, we should be notified in order to re-evaluate our design parameters and estimates, and to provide additional recommendations if necessary.

Conventional strip and spread footings may be used to support the proposed residences. Foundations should not be installed on topsoil, disturbed native soils, undocumented fill, debris, combination soils, frozen soil, or in ponded water. If foundation soils become disturbed during construction they should be removed or recompacted until firm.

Foundations may be constructed entirely on undisturbed, uniform, native gravel soils, or entirely on a minimum 18 inches of properly placed and compacted structural fill placed on undisturbed native soils. If soil conditions which vary from those observed in the test pits are encountered in foundation excavations, an engineer from Earthtec should observe the soil conditions and make additional recommendations if necessary, particularly in those areas of the site which we

were unable to explore. For design of conventional strip and spread footings, we recommend the following parameters:

Minimum embedment for frost protection:	30 inches
Minimum strip footing width:	20 inches
Minimum spot footing width:	30 inches
Maximum allowable net bearing pressure:	2,000 psf
Bearing pressure increase for transient loading:	33 percent

Structural fill used below foundations should extend laterally a minimum of 6 inches for every 12 vertical inches of structural fill placed. For example, if 18 inches of structural fill are required to bring the excavation to footing grade, the structural fill should extend laterally a minimum of 9 inches beyond the edge of the footings.

10.2 Estimated Settlement

If the proposed foundations are properly designed and constructed using the parameters provided above, total settlement for non-earthquake conditions is estimated not to exceed one inch. Differential settlement is anticipated to be one-half of the total settlement over a 25-foot length of foundation. Additional movements could occur during an earthquake due to ground shaking.

11.0 SUBSURFACE DRAINAGE

According to Section R405 of the 2003 International Residential Code, "Drains shall be provided around all concrete or masonry foundations that retain earth and enclose habitable or usable spaces located below grade." An exception is allowed when the foundation is installed on well drained ground consisting of Group 1 soils. These soils include those defined by the Unified Soil Classification System as GW, GP, SW, SP, GM, and SM. The subsurface native soils observed in the test pits are Group 1 soils.

12.0 FLOOR SLABS

To facilitate construction, act as a capillary break, and aid in distributing floor loads we recommend that all at-grade slabs and exterior flatwork be underlain by four inches of free-draining granular material such as "pea" gravel or three-quarters to one-inch minus clean gravel supported on competent native soils or structural fill.

To help control normal shrinkage and stress cracking the floor slabs should have the following features:

1. Adequate reinforcement for the anticipated floor loads with the reinforcement continuous through interior floor joints;
2. Frequent crack control joints; and
3. Non-rigid attachment of the slabs to foundation and bearing walls.

Special precautions should be taken during placement and curing of all concrete slabs and flatwork. Excessive slump (high water-cement ratios) of the concrete and/or improper finishing and curing procedures used during hot or cold weather conditions may lead to excessive shrinkage, cracking, spalling, or curling of slabs. We recommend all concrete placement and curing operations be performed in accordance with American Concrete Institute (ACI) codes and practices.

13.0 MOISTURE CONTROL AND SURFACE DRAINAGE

Precautions should be taken during and after construction to reduce the potential for saturation of foundation soils. We recommend that the following precautions be taken at this site:

1. Adequate compaction of foundation backfill should be provided i.e. a minimum of 90% of ASTM D-1557. **Water consolidation methods should not be used.**

2. The ground surface should be graded to drain away from the residence in all directions. We recommend a minimum fall of 6 inches in the first 10 feet.

3. Roof runoff should be collected in rain gutters with down spouts designed to discharge well outside of the backfill limits, or at least 10 feet from foundations, whichever is greater.

4. Sprinklers should be aimed away foundation walls. Sprinkler systems should be well maintained, checked for leaks frequently, and repaired promptly.

5. Any additional precautions which may become evident during construction.

14.0 PAVEMENT DESIGN

We anticipate that asphalt concrete paved streets will be constructed within this development to serve the residences. We have based our design on the near surface gravel soils encountered in the test pits and conservatively estimate a California Bearing Ratio (CBR) value of 10 for these soils.

We assume that traffic volumes (consisting mostly of cars and pickup trucks, a daily school bus, some delivery trucks, and a weekly garbage truck) will be relatively light, about 100 vehicles per day or less. We have also included some construction traffic (2 dump trucks and 2 concrete trucks per day. We have prepared a pavement section design based on the assumed CBR value, traffic assumptions as given above, a design life of 20 years, and the site grading recommendations presented in this report. Based on these parameters and the procedures outlined in the *AASHTO Guide for Design of Pavement Structures (1993)*, we recommend the minimum asphalt pavement sections presented in the table below.

Table No. 2: Pavement Section Design

ASPHALT THICKNESS (in)	COMPACTED ROADBASE THICKNESS (in)	COMPACTED SUBBASE THICKNESS (in)
3.5	6.0	0.0

The pavement section recommended is not intended to support heavy semi trucks. All base material and asphalt should conform to local or UDOT requirements regarding thickness, gradation, oil content, and any other requirements pertaining to the project. We recommend that all roadbase and subbase be properly processed, moisture conditioned, and compacted to a minimum of 95% of the maximum dry density as determined by ASTM-D 1557. All asphalt should be compacted to a minimum of 95% of the laboratory Marshal mix design density.

15.0 GENERAL CONDITIONS

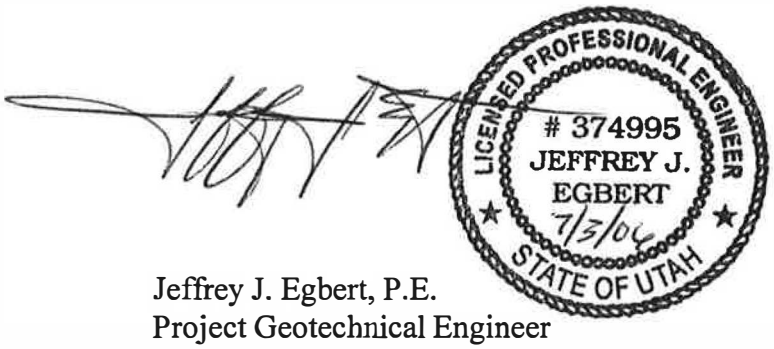
The exploratory data presented in this report was collected to provide geotechnical design recommendations for this project. The test pits may not be indicative of subsurface conditions outside the study area or between points explored and thus have a limited value in depicting subsurface conditions for contractor bidding. Variations from the conditions portrayed in the test pits may occur and which may be sufficient to require modifications in the design. If during construction, conditions are different than presented in this report, please advise us so that the appropriate modifications can be made.

The geotechnical study as presented in this report was conducted within the limits prescribed by our client, with the usual thoroughness and competence of the engineering profession in the area. No other warranty or representation, either expressed or implied, is intended in our proposals, contracts or reports.

We appreciate the opportunity of providing our services on this project. If we can answer questions or be of further service, please call.

Respectfully;

EARTHTEC TESTING AND ENGINEERING, P.C.

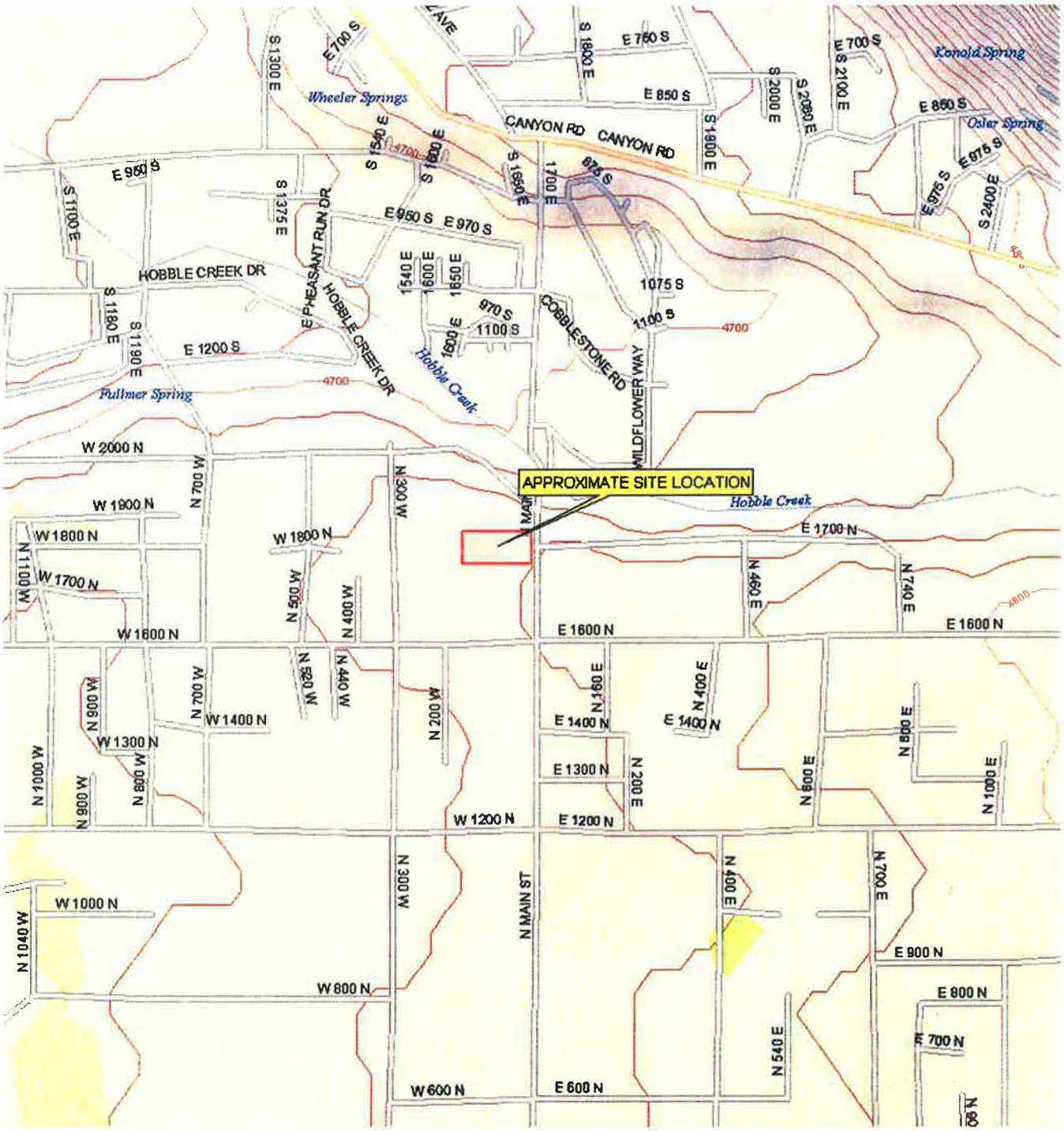


Jeffrey J. Egbert, P.E.
Project Geotechnical Engineer

William G. Turner, P.E.
Senior Geotechnical Engineer

VICINITY MAP

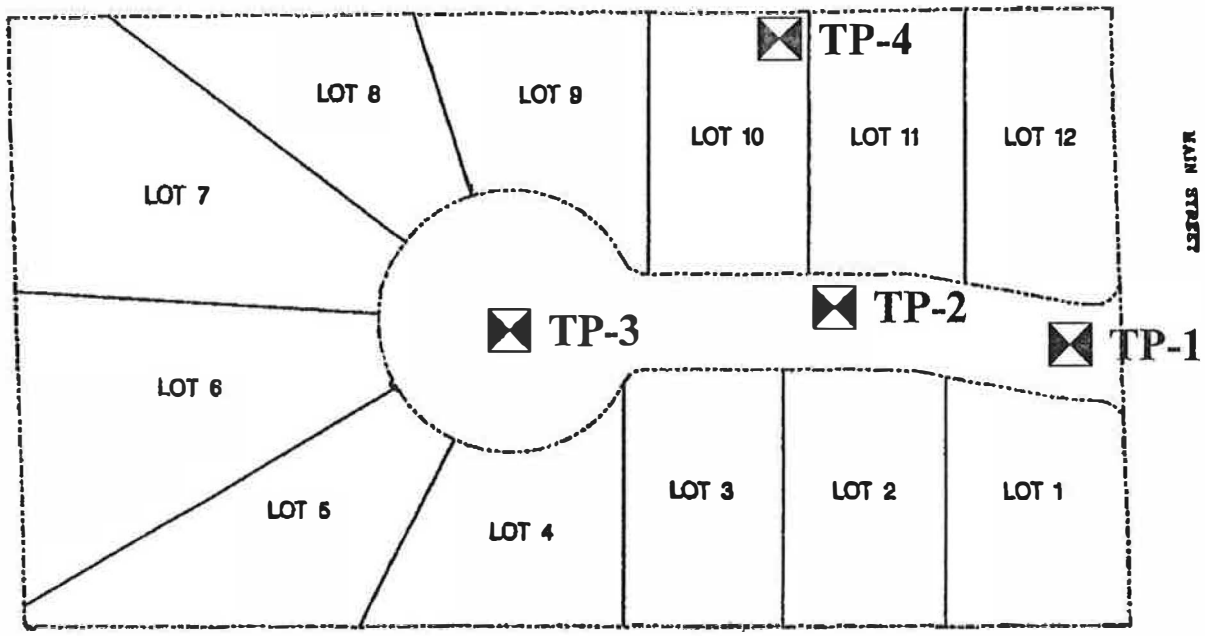
CARNESECCA ORCHARD ESTATES



PROJECT NO.: 061567	Earthtec Testing and Engineering, L.C.	**FIGURE NO.:** 1

SITE PLAN & LOCATION OF TEST PITS

CARNESECCA ORCHARD ESTATES



PROJECT NO.: 061567	Earthtec	FIGURE NO.: 2

TEST PIT LOG
NO.: TP-1

PROJECT: Carnesecca Orchard Estates
CLIENT: Chris Shurian
LOCATION: Refer to Figure 2.
OPERATOR: Halls
EQUIPMENT: RTB
DEPTH TO WATER; INITIAL ▽ :

PROJECT NO.: 061567
DATE: 06/13/06
ELEVATION: NM
LOGGED BY: D.S.
AT COMPLETION ▼ :

Depth (Ft.)	Graphic Log	USCS	Description	Samples	TEST RESULTS: Dry Dens. (pcf)	Water Cont. (%)	PI	LL	Gravel (%)	Sand (%)	Fines (%)	Other Tests
0			TOPSOIL: Silt with gravel, some sand and cobbles, roots, moist, dark brown.									
2		GP	POORLY GRADED GRAVEL with sand, cobbles, trace silt, dense, moist, brown.	X								
6			Medium dense at 6 feet.	X		6			66	30	4	
8				X								
9			Bottom at approximately 9 feet.									

Notes: No groundwater encountered.

Tests Key
CBR = California Bearing Ratio
C = Consolidation
R = Resistivity
DS = Direct Shear
SS = Soluble Sulfates
UC = Unconfined Compressive Strength

PROJECT NO.: 061567

Earthtec Testing and Engineering, P.C.

FIGURE NO.: 3

LOG OF TESTPIT 061567.GPJ EARTHTEC.GDT 6/29/06

TEST PIT LOG
NO.: TP-2

PROJECT: Carnesecca Orchard Estates
CLIENT: Chris Shurian
LOCATION: Refer to Figure 2.
OPERATOR: Halls
EQUIPMENT: RTB
DEPTH TO WATER; INITIAL ▽ :

PROJECT NO.: 061567
DATE: 06/13/06
ELEVATION: NM
LOGGED BY: D.S.
AT COMPLETION ▼ :

Depth (Ft.)	Graphic Log	USCS	Description	Samples	TEST RESULTS: Dry Dens. (pcf)	Water Cont. (%)	PI	LL	Gravel (%)	Sand (%)	Fines (%)	Other Tests
0			TOPSOIL: Silt with sand and gravel, some cobbles, roots, moist, dark brown.									
1												
2		GM	SILTY GRAVEL with sand, cobbles, dense, moist, brown.									
3				X								
4		GP-GM	POORLY GRADED GRAVEL with silt and sand, cobbles, dense, moist, brown.									
5												
6				X								
7												
8												
9		SP-SM	POORLY GRADED SAND with silt and gravel, loose, moist, brown.	X		7			39	51	10	
10												
11			Bottom at approximately 10 feet.									
12												

Notes: No groundwater encountered.

Tests Key
CBR = California Bearing Ratio
C = Consolidation
R = Resistivity
DS = Direct Shear
SS = Soluble Sulfates
UC = Unconfined Compressive Strength

PROJECT NO.: 061567

Earthtec Testing and Engineering, PC

FIGURE NO.: 4

LOG OF TESTPIT 061567.GPJ EARTHTEC.GDT 6/29/06

TEST PIT LOG
NO.: TP-3

PROJECT: Carnesecca Orchard Estates
CLIENT: Chris Shurian
LOCATION: Refer to Figure 2.
OPERATOR: Halls
EQUIPMENT: RTB
DEPTH TO WATER; INITIAL ▽ :

PROJECT NO.: 061567
DATE: 06/13/06
ELEVATION: NM
LOGGED BY: D.S.
AT COMPLETION ▼ :

Depth (Ft.)	Graphic Log	USCS	Description	Samples	TEST RESULTS: Dry Dens. (pcf)	Water Cont. (%)	PI	LL	Gravel (%)	Sand (%)	Fines (%)	Other Tests
0			TOPSOIL: Silt with sand and gravel, some cobbles, moist, dark brown.									
1												
2		GP	POORLY GRADED GRAVEL with sand, cobbles, trace silt, dense, moist, brown.									
3				X		4			69	28	3	
4												
5				X								
6												
7												
8				X								
9												
10												
11			Bottom at approximately 10 feet.									
12												

Notes: No groundwater encountered.

Tests Key
CBR = California Bearing Ratio
C = Consolidation
R = Resistivity
DS = Direct Shear
SS = Soluble Sulfates
UC = Unconfined Compressive Strength

PROJECT NO.: 061567

Earthtec Testing and Engineering, P.C.

FIGURE NO.: 5

LOG OF TESTPIT 061567.GPJ EARTHTEC.GDT 6/29/06

TEST PIT LOG
NO.: TP-4

PROJECT: Carnesecca Orchard Estates
PROJECT NO.: 061567
CLIENT: Chris Shurian
DATE: 06/13/06
LOCATION: Refer to Figure 2.
ELEVATION: NM
OPERATOR: Halls
LOGGED BY: D.S.
EQUIPMENT: RTB
DEPTH TO WATER; INITIAL ▽ :
AT COMPLETION ▼ :

Depth (Ft.)	Graphic Log	USCS	Description	Samples	TEST RESULTS: Dry Dens. (pcf)	Water Cont. (%)	PI	LL	Gravel (%)	Sand (%)	Fines (%)	Other Tests
0			TOPSOIL: Silt with sand and gravel, some cobbles, moist, dark brown.									
2		GM	SILTY GRAVEL with sand, cobbles, dense, moist, brown.	X								
4		GP-GM	POORLY GRADED GRAVEL with sand, cobbles, trace silt, dense, moist, brown.	X		3			65	32	3	
11			Bottom at approximately 11 feet.	X								

Notes: No groundwater encountered.

Tests Key
CBR = California Bearing Ratio
C = Consolidation
R = Resistivity
DS = Direct Shear
SS = Soluble Sulfates
UC = Unconfined Compressive Strength

PROJECT NO.: 061567

Earthtec Testing and Engineering, PC

FIGURE NO.: 6

LOG OF TESTPIT 061567.GPJ EARTHTEC.GDT 6/29/06

LEGEND

PROJECT: Carnesecca Orchard Estates **DATE:** 06/13/06
CLIENT: Chris Shurian **LOGGED BY:** D.S.

UNIFIED SOIL CLASSIFICATION SYSTEM

MAJOR SOIL DIVISIONS			USCS SYMBOL	TYPICAL SOIL DESCRIPTIONS
COARSE GRAINED SOILS (More than 50% retaining on No. 200 Sieve)	GRAVELS (More than 50% of coarse fraction retained on No. 4 Sieve)	CLEAN GRAVELS (Less than 5% fines)	GW	Well Graded Gravel, May Contain Sand, Very Little Fines
			GP	Poorly Graded Gravel, May Contain Sand, Very Little Fines
		GRAVELS WITH FINES (More than 12% fines)	GM	Silty Gravel, May Contain Sand
			GC	Clayey Gravel, May Contain Sand
	SANDS (50% or more of coarse fraction passes No. 4 Sieve)	CLEAN SANDS (Less than 5% fines)	SW	Well Graded Sand, May Contain Gravel, Very Little Fines
			SP	Poorly Graded Sand, May Contain Gravel, Very Little Fines
		SANDS WITH FINES (More than 12% fines)	SM	Silty Sand, May Contain Gravel
			SC	Clayey Sand, May Contain Gravel
FINE GRAINED SOILS (More than 50% passing No. 200 Sieve)	SILTS AND CLAYS (Liquid Limit less than 50)		CL	Lean Clay, Inorganic, May Contain Gravel and/or Sand
			ML	Silt, Inorganic, May Contain Gravel and/or Sand
			OL	Organic Silt or Clay, May Contain Gravel and/or Sand
	SILTS AND CLAYS (Liquid Limit Greater than 50)		CH	Fat Clay, Inorganic, May Contain Gravel and/or Sand
			MH	Elastic Silt, Inorganic, May Contain Gravel and/or Sand
			OH	Organic Clay or Silt, May Contain Gravel and/or Sand
HIGHLY ORGANIC SOILS			PT	Peat, Primarily Organic Matter

SAMPLER DESCRIPTIONS

- SPLIT SPOON SAMPLER (1 3/8 inch inside diameter)
- MODIFIED CALIFORNIA SAMPLER (2½ inch outside diameter)
- SHELBY TUBE (3 inch outside diameter)
- BLOCK SAMPLE
- BAG/BULK SAMPLE

WATER SYMBOLS

- ▽ Water level encountered during field exploration
- ▼ Water level encountered at completion of field exploration

NOTES:
1. The logs are subject to the limitations, conclusions, and recommendations in this report.
2. Results of tests conducted on samples recovered are reported on the logs and any applicable graphs.
3. Strata lines on the logs represent approximate boundaries only. Actual transitions may be gradual.
4. In general, USCS symbols shown on the logs are based on visual methods only: actual designations (based on laboratory tests) may vary.

PROJECT NO.: 061567 | Earthtec Testing and Engineering, PC | **FIGURE NO.:** 7